CS-24-266

SECTION 00 63 63

CHANGE ORDER REQUEST FORM

PROJECT ITB NC23-064 / CIP008 / PDCII Contract No. CM3680 No. 4

DATE OF ISSUANCE 2/25/25 EFFECTIVE DATE 2/25/25

NASSAU COUNTY BOARD OF COUNTY COMMISSIONERS

COUNTY Contract No.: CM CM3680

CONTRACTOR- Reeves Construction Co. ENGINEER / ARCHITECT- James Cuneo, CEI PA

Description of changes:

Contractor submitted RFI #10 for lack of Base Material under pavement, see attached. The County obtained testing from a third party (ECS) to confirm finding and provide info for repair, see attached Geo-tech Report and Recommendations. The County has chosen Option 1 for the repair and CEI concurs with the pricing and requested 1 day for Option 1.

Attachments: (List documents supporting change) Reeves CO #002 Quote Ltr, EOR Response RFI #10, ECS Report

CHANGE IN CONTRACT PRICE:	CHANGE IN CONTRACT TIMES:
Original Contract Price $ 7,549,148.88	Original Contract Times Substantial Completion: (420) 9/25/2025 (days and dates) Ready for Final Payment: (465) 11/9/2025 (days and dates)
Net change from previous Change Orders No. 0 to No. 3 $ 58,267.56	Net change from previous Change Orders No. 1 to No. 3 38 (days)
Contract Price prior to this Change Order $ 7,607,416.44	Contract Times Prior to This Change Order Substantial Completion: (458) 11/2/2025 (days and dates) Ready for Final Payment: (503) 12/17/2025 (days and dates)
Net Increase (decrease) of this Change Order $ 61,471.00	Net Increase (decrease) of this Change Order 1 (Increase) (days)
Contract Price with all approved Change Orders $ 7,668,887.44	Contract Times with all approved Change Orders Substantial Completion: (459) 11/3/2025 (days and dates) Ready for Final Payment: (504) 12/18/2025 (days and dates)

RECOMMENDED:

By: James B Cuneo

Digitally signed by James B Cuneo
DN: CN=James B Cuneo, SN=Qualifier=A0410C0000018E3465DD3500039B7E, C=Unclassified, C=US
Date: 2025.02.28 07:55:39-05'00'

Engineer/Architect (Authorized Signature)

Date: 2/28/2025

APPROVED:

By: Raymond Allen

COUNTY (Authorized Signature)

Date: 3/6/2025

ACCEPTED:

By: Clyde Cross

Digitally signed by Clyde Cross
DN: C=US, E=ccross@reevescc.com, CN=Clyde Cross
Date: 2025.02.28 08:11:11-05'00'

Contractor (Authorized Signature)

Date: 2/28/2025

CHANGE ORDER APPROVAL FORM

PROJECT: PDCII - Chester Rd & Pages Dairy Rd Intersection Improvements

CHANGE ORDER NUMBER: 04

DATE: 2/25/2025

CONTRACT NUMBER: CM3680

PURCHASE ORDER NUMBER (IF APPLICABLE): 24000508

TO CONTRACTOR: Reeves Construction Co.

Description:
Contractor submitted RFI #10 for lack of Base Material under pavement, see attached. The County obtained testing from a third party (ECS) to confirm finding and provide info for repair, see attached Geo-tech Report and Recommendations. The County has chosen Option 1 for the repair and CEI concurs with the pricing and requested 1 day for Option 1.

Reason for Change Order:

For additional materials based on contract unit quantities and necessary time extension.

Original Contract Sum……	$	7,549,148.88
Net Change by Previous Change Order/Supplemental Agreement.	$	58,267.56
Contract Sum Prior to This Change Order……..	$	7,607,416.44
Amount of this Change Order (Add)……..	$	61,471.00
New Contract Sum Including this Change Order…….	$	7,668,887.44

The Contract Time will be increased or decreased (select one) 1 days
Previous Substantial Completion: 11/2/2025 Previous Final Completion: 12/17/2025
New Substantial Completion: **11/3/2025** New Final Completion: **12/18/2025**

APPROVED BY: Robert Companion DATE: 3/6/2025
Department Head/Managing Agent

APPROVED BY: Tamara Kilmore DATE: 3/11/2025
Procurement

APPROVED BY: Chris Lacambra DATE: 3/7/2025
Office of Management and Budget

APPROVED BY: Denise C. May, Esq., BCS DATE: 3/13/2025
County Attorney

APPROVED BY: DATE: 3/13/2025
County Manager

APPROVED BY: N/A DATE:
Chairman

ATTEST: N/A DATE:
Mitch L. Keiter, Clerk of Courts

Account No(s). 63470541-563100 PDCII

REEVES
A COLAS COMPANY

November 4, 2024

Mr. James Cuneo, PWM
Project Administrator
CSI Geo, Inc.
2394 St. Johns Bluff Road, South, Suite 200
Jacksonville, FL 32246

Re: Chester Road & Pages Dairy Road Intersection Improvements
Bid No.: NC23-064
CIP No.: 008
Contract: CM3680

Subject: CR #002.0 (Pages Dairy Rebuild 240+25 to 243+75)

Dear Mr. Cuneo:

Per your request we offer the following options to rebuild Pages Dairy Road at the location above.

- Option #1 (7” of Black Base) - $61,471
- Option #2 (6” Limerock with 12” Stabilization) - $66,512

Both of the options would require closing down Pages dairy Road for the work. Option 1 should be able to be done in one day and Option 2 should be able to be done in five days.

If any additional information is required, please feel free to contact us.

Sincerely
Reeves Construction Company

Clyde Cross

Clyde Cross
Area Manager

Reeves Construction Company
249 Industry Place
St. Augustine, FL 32095
(904) 824-9901 Phone

REQUEST FOR INFORMATION

To:	**Mr. James Cuneo, PWM**	Location:	CIP008 / PDCII Contract CM3680
From:	**Clyde Cross**		

RFI No.	**10**		
Project	Chester Rd. & Pages Dairy Rd. Intersection Improvements	Date:	09/30/24
P.O. No.:		Bid No.	
Area Involved	Pages Dairy Road from 240+50 to 243+00		
Drawing No.			

Information Requested:
We had ECS take 4 pavement cores in Pages dairy and there is no base under an average of 2.93" of Asphalt. Please advise.

Submitted By: **Clyde Cross**

Date Answer Requested By: 10/02/24

Reply:
The report from Terracon mentions 6 to 9 inches of sandy subgrade material stabilized with limerock fragments below the asphalt, but do not consider it base material. To get an acceptable structural number for the Pages Dairy Road area in question I would recommend the addition of upgraded base material. I would recommend 6" limerock with 12" stabilization or 3.5" black base with 12" stabilization. As an option to construct without stabilization, could use 7" black base.

Reply By: **Danny Waltermyer**

Date: **2/18/2025**

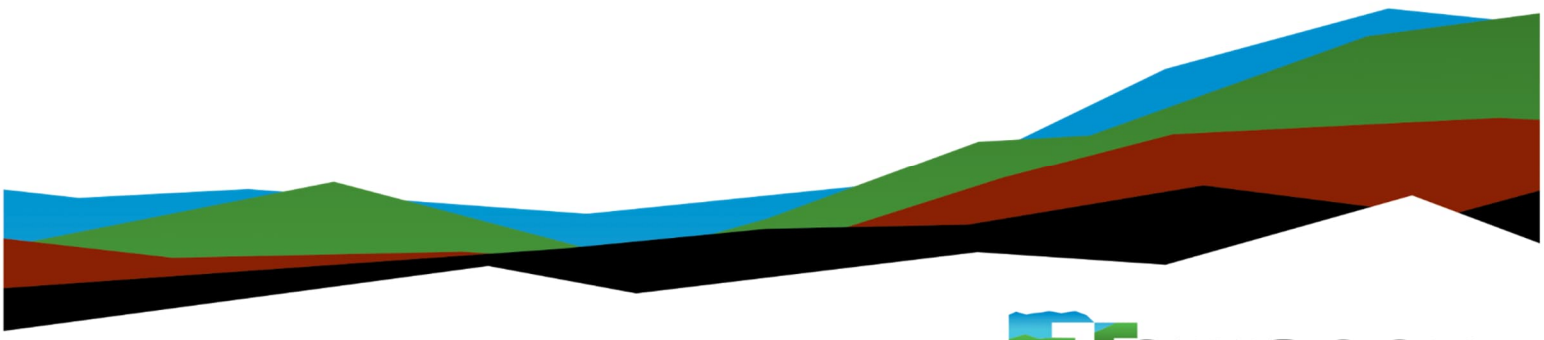
Pages Dairy Road – Pavement Cores

Geotechnical Engineering Report

December 11, 2024 | Terracon Project No. EQ245111

Prepared for:

Nassau County Board of County Commissioners
96161 Nassau Place
Yulee, Florida 32097



8001 Baymeadows Way – Suite 1
Jacksonville, Florida 32256
P (904) 900-6494
F (904) 268-5255
Terracon.com

December 11, 2024

Nassau County Board of County Commissioners
96161 Nassau Place
Yulee, Florida 32097

Attn: Mr. George Murray
P: (904) 530-6373
E: GMurray@NassauCountyFl.com

Re: Geotechnical Engineering Report
Pages Dairy Road – Pavement Cores
Chester Road from David Hallman Parkway to North of Pages Dairy Road
Yulee, Nassau County, Florida
Terracon Project No. EQ245111
Nassau County Contract No. CM3583-WA03

Dear Mr. Murray:

Terracon Consultants, Inc. (Terracon) is pleased to present this geotechnical engineering report for the above-mentioned project. Our geotechnical services for the project were performed in general accordance with Work Authorization #3 between Nassau County and Terracon, dated October 30, 2024, and our Proposal for Geotechnical Services, dated October 15, 2024.

The purpose of this exploration is to evaluate the subsurface conditions and pavement section encountered in an approximate 250-foot section of Pages Dairy Road as part of proposed roadway improvements. This report documents the pavement section and subsurface conditions encountered.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely,

Terracon Consultants, Inc.

Thomas D. Hallahan
Project Manager
Geotechnical Services

JOHN B. KIMBERLY IV LICENSE No 49866 STATE OF FLORIDA PROFESSIONAL ENGINEER

John B Kimberly

Digitally signed by John B Kimberly
Date: 2024.12.11 11:50:33 -05'00'

John B. Kimberly IV, P.E.
Senior Consultant
FL Registration No. 49866

This document has been digitally signed and sealed by John B. Kimberly IV, PE on the date adjacent to the seal.
Printed copies of this document are not considered signed and sealed, and the signature must be verified on any electronic copies

Table of Contents

REPORT SUMMARY i
Purpose and Scope 2
Project Description 3
Site Conditions 4
Soil Survey 4
U.S.G.S. Topographic Quadrangle Map 4
Geotechnical Characterization 5
Typical Subsurface Profile 5
Groundwater Conditions 5
Laboratory Testing 6
General Comments 6

Attachments

Appendix A Field Exploration
Appendix B Laboratory Testing
Appendix C Supporting Information

Note: This report was originally delivered in a web-based format. Blue Bold text in the report indicates a referenced section heading. The PDF version also includes hyperlinks which direct the reader to that section and clicking on the terracon logo will bring you back to this page. For more interactive features, please view your project online at client.terracon.com.

Refer to each individual Attachment for a listing of contents.

REPORT SUMMARY

Topic [1]	Overview Statement [2]
Project Description	A geotechnical exploration was performed for the proposed improvements along Pages Dairy Road, in Yulee in Nassau County, Florida. The exploration consisted of the following: ■ Four pavement cores through existing asphalt pavement at four locations along the existing roadway. ■ Four 5-feet hand augers beneath the asphalt for sampling of any base material and surficial soils
Typical Subsurface Profile	■ In general, the pavement cores initially encountered approximately 2.5-inches of asphaltic concrete. ■ The hand augers, performed beneath the asphalt, initially encountered a layer of approximately 6 to 9-inches of sandy subgrade material stabilized with limerock fragments. (Unified Soil classifications of SP-SM, SM). This was followed by sandy soils of varying fines content (SP, SP-SM) to the termination depth of approximately 5-feet below the existing roadway. ■ No base material was encountered beneath the existing pavement.
Groundwater	Groundwater was encountered from approximate depths of 4.5 and 5 feet in two of the pavement cores/hand augers, and was not encountered to the termination depth of approximately 5-feet in the other two.
General Comments	This section contains important information about the limitations of this geotechnical engineering report.

1. If the reader is reviewing this report as a pdf, the topics above can be used to access the appropriate section of the report by simply clicking on the topic itself.
2. This summary is for convenience only. It should be used in conjunction with the entire report for design purposes.

Geotechnical Engineering Report

Pages Dairy Road – Pavement Cores

Chester Road from David Hallman Parkway to North of Pages Dairy Road
Yulee, Nassau County, Florida
Nassau County Contract No: CM3583-WA03
Terracon Project No. EQ245111
December 11, 2024

Purpose and Scope

This report presents the results of our subsurface exploration and geotechnical engineering services performed for the proposed roadway improvements. The purpose of these services is to provide a geotechnical engineering report to include:

- Stratified pavement core/hand auger logs based on visual soil classification
- Groundwater levels observed during and after the completion of drilling activities
- Site Location and Exploration Plans
- Subsurface exploration and laboratory testing procedures
- Description of subsurface conditions and pavement section encountered
- Results of all laboratory testing performed
- Photographs of pavement cores retrieved

The geotechnical engineering Scope of Services for this project included the following:

- Review of United States Geological Survey (USGS) Quadrangle Map for the site proximity.
- Review of United States Department of Agriculture (USDA) Soil Conservation Service (SCS) Soil Survey Map.
- Mobilizing a drill rig and crew to the site and performing four pavement cores and hand augers in the existing roadway along a section of Pages Dairy Road
- Description of Field Exploration Procedures
- Description of Laboratory Testing Procedures
- Laboratory index testing on select samples from borings.
- Preparation of this report.

Maps and location plans showing the project site, along with the core profiles and field exploration procedures are shown in the Appendix A. The results of the laboratory

testing and the testing procedures performed on soil samples obtained from the site during the field exploration, are included on the log sheets and summarized in Appendix B.

Project Description

Our initial understanding of the project was provided in our proposal and was discussed with the design team in various correspondence occurring during the course of the project. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

Item	Description
Proposed Improvements	The proposed improvements include roadway widening and the construction of a paved shoulder along Pages Dairy Road as it nears the intersection with Chester Road. It is part of the larger Nassau County Engineering project; Chester Road, from David Hallman Parkway to North of Pages Dairy Road.
Project Location	The project is a 250-foot roadway section, located on Pages Dairy Road just west of the intersection with Chester Road, in Yulee, Florida.
Current Ground Cover	Pages Dairy Road is a two-lane rural highway, with turn lanes at the intersection with Chester Road. The posted speed limit through the project site is 45 MPH.
Existing Topography	The site topography is relatively flat within the right-of-way of the roadway, with drainage swales on both sides and overhead powerlines running on the north side of the roadway. According to topographic quadrangle maps of the area, the natural ground surface elevation of the general site area ranges from about +25 to +30 feet (Assumed NAVD 88 datum).
Prior Land Use	Review of historical aerial photographs (ref. Google Earth) indicate that the project area had been relatively unchanged from at least 1988 until about 2011, when the powerline easement was installed on the north side of the roadway. Turn lanes were added approaching the intersection with Chester Road sometime between 2011 and 2013.

Site Conditions

The following description of site conditions is derived from our site visit in association with the field exploration and our review of publicly available geologic and topographic maps.

Soil Survey

The Soil Survey for Nassau County, Florida, as prepared by the United States Department of Agriculture (USDA), Soil Conservation Service (now renamed the Natural Resource Conservation Service - NRCS), identifies two soil types at the subject site as shown below.

The Web Soil Survey (WSS) map of the project area was reviewed and a map encompassing the project area is included as Soils Map in Appendix A. The WSS presents shallow (typically upper 80 inches) soil stratification information produced and compiled by the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS). Soils Map identifies the soil map units documented by the NRCS in the project area. The typical soil types and estimated seasonal high groundwater levels for the map units, are given in the following table.

SUMMARY OF SOILS IN PROJECT VICINITY – FROM NRCS WEB SOIL SURVEY

Map Unit No. And Name	Stratification				Estimated Season High Groundwater Level (feet)
	Depth Range (inches)	Unified Soil Classification	AASHTO Soil Classification	Permeability (in/hour)	
24 Kingsferry Fine Sand	0 – 34 34 – 67 67 – 80	SP, SP-SM SP-SM, SM SP-SM, SM	A-3 A-3, A-2-4 A-2-4	2.0 – 6.0 2.0 – 6.0 0.2 – 0.6	0.0 – 0.5 Perched Dec - Oct
27 Ridgewood Fine Sand 0 to 5% slopes	0 – 7 7 – 80	SP, SP-SM SP, SP-SM	A-3 A-3	6.0 – 20 6.0 - 20	2.0 – 3.5 Apparent Jun - Oct

U.S.G.S. Topographic Quadrangle Map

Based on the United States Geological Survey (USGS) “St Marys, Florida,” topographic quadrangle maps (issued in 1993) the natural ground surface in the proposed project area appears to be between approximately +25 to +30 feet NGVD. Excerpts of the USGS Quadrangle Map of the project area are shown as Topographic Vicinity Map in the Appendix.

Geotechnical Characterization

Typical Subsurface Profile

The subsurface soils encountered in the cores consisted of the following:

- All pavement core locations initially encountered approximately 2.3 to 2.6 inches of asphaltic concrete.
- The pavement was underlain by approximately 6 to 9 inches of sand with silt or silty sand (SP-SM, SM), stabilized with trace to few, coarse to fine limerock fragments.
- Beneath the stabilized subgrade layer, the augers encountered layers of fine sand and fine sand with silt (SP, SP-SM) to the termination depth of approximately 5-feet below the existing pavement.
- No aggregate base course, typically Florida limerock in this area, was encountered in the four pavement cores.

For a subsurface profile at each boring, please refer to the Coring Logs in Appendix A. Samples of the encountered soils were placed in glass jars that were transported to our laboratory for review by a Geotechnical Engineer. The soil descriptions presented on the coring logs are based on a visual/manual classification procedure in general accordance with AASHTO and USCS standards, as well as laboratory data when available. Descriptions of both AASHTO and USCS soil classification systems are shown in Appendix C.

The coring profiles provide detailed descriptions of the subsurface conditions encountered at each boring location. When reviewing the coring logs and the subsurface profiles, it should be understood that soil conditions might vary between and away from coring locations. Stratification boundaries on logs represent the approximate depth of changes in soil types; the transition between materials may be gradual.

Groundwater Conditions

The groundwater level was measured in the borings at the time of our exploration, and will fluctuate due to seasonal climatic variations, previous rainfall, construction operations, surrounding development, and other interrelated factors. The groundwater level was encountered at depths of approximately 4.5 and 5 feet below the existing ground at the two western-most pavement cores; C240+50 & C242+00 respectively. Groundwater was not encountered to the termination depth of approximately 5-feet in the other two pavement cores; C242+50 & C243+00. The measured groundwater at each core location, where encountered, is indicated on the Coring Logs in Appendix A.

It should be recognized that fluctuations of the groundwater table will occur due to seasonal variations in the amount of rainfall, runoff, and other factors not evident at the time the borings were performed. Therefore, groundwater levels during construction or at other times in the future may be higher or lower than the levels indicated on the boring profile sheet. It should be noted that changes in on-site or adjacent off-site surface hydrology and subsurface drainage could have significant effects on the seasonal high groundwater levels in the project area.

Laboratory Testing

The results of laboratory testing, including moisture content and grain size distribution, are presented on the Summary of Laboratory Testing Results tables included as Exhibit B-2, as well as the Coring logs in Appendix A. A description of laboratory testing procedures is presented as Exhibit B-1 in Appendix B.

General Comments

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials, or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client and is not intended for third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly affect excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety and cost estimating including excavation support and dewatering requirements/design are the responsibility of others. Construction and site development have the potential to affect adjacent properties. Such impacts can include damage due to vibration, modification of groundwater/surface water flow during construction, foundation movement due to undermining or subsidence from excavation, as well as noise or air quality concerns. Evaluation of these items on nearby properties are commonly associated with contractor means and methods and are not addressed in this report. The owner and contractor should consider a preconstruction/precondition survey of surrounding development. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

Appendix A

U.S.G.S Topographic Map

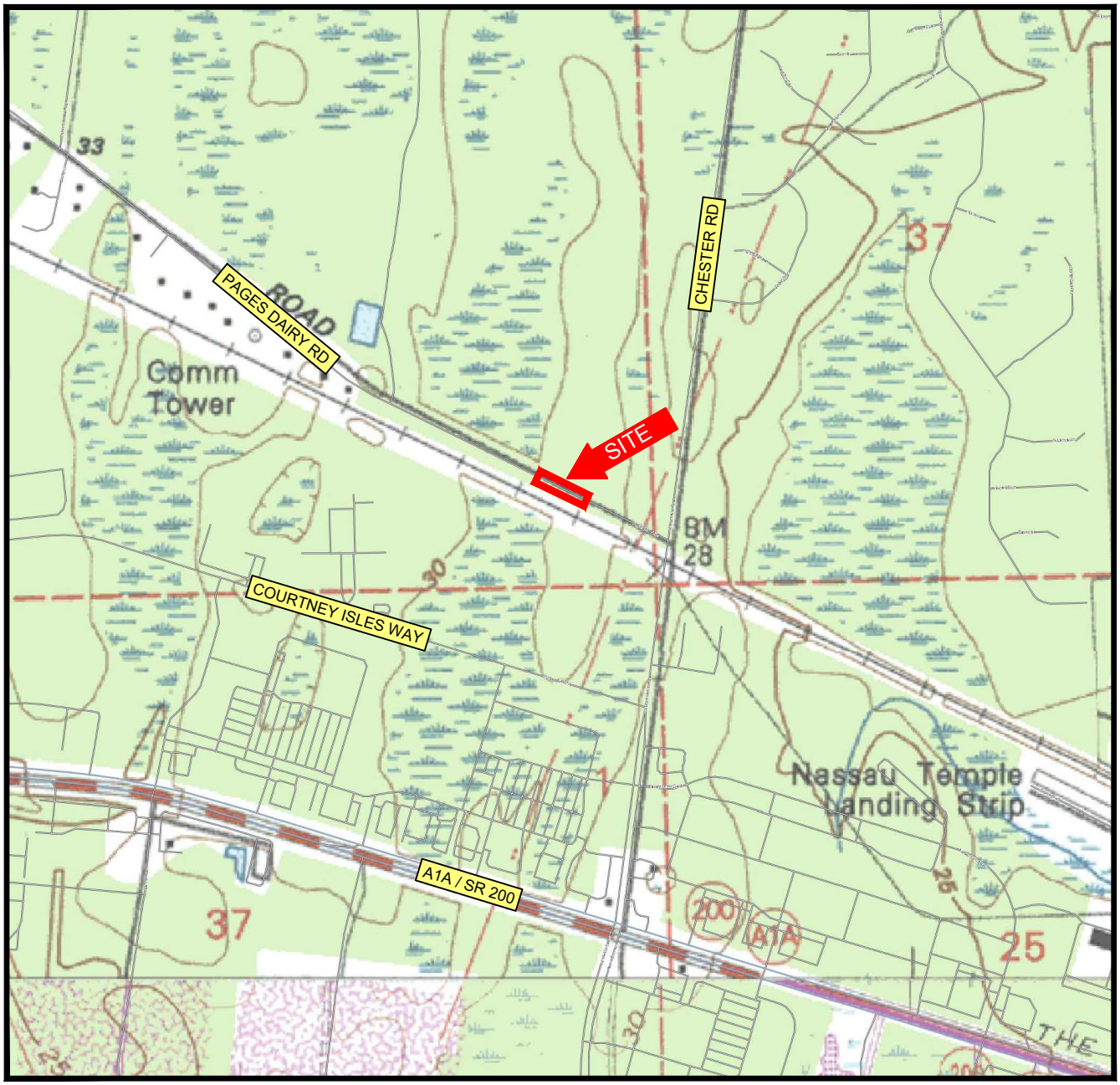
U.S.D.A. Soils Map

Exploration Location Plan

Field Exploration Procedures

Coring Logs (4 pages)

UNITED STATES – DEPARTMENT OF THE INTERIOR – GEOLOGICAL SURVEY



SCALE 1"=1000'

FEET

500 0 500 1000 1500 2000 2500 3000 3500 4000 4500 5000

ST MARYS, FLORIDA
ISSUED: 1993
7.5 MINUTE SERIES (QUADRANGLE)

N

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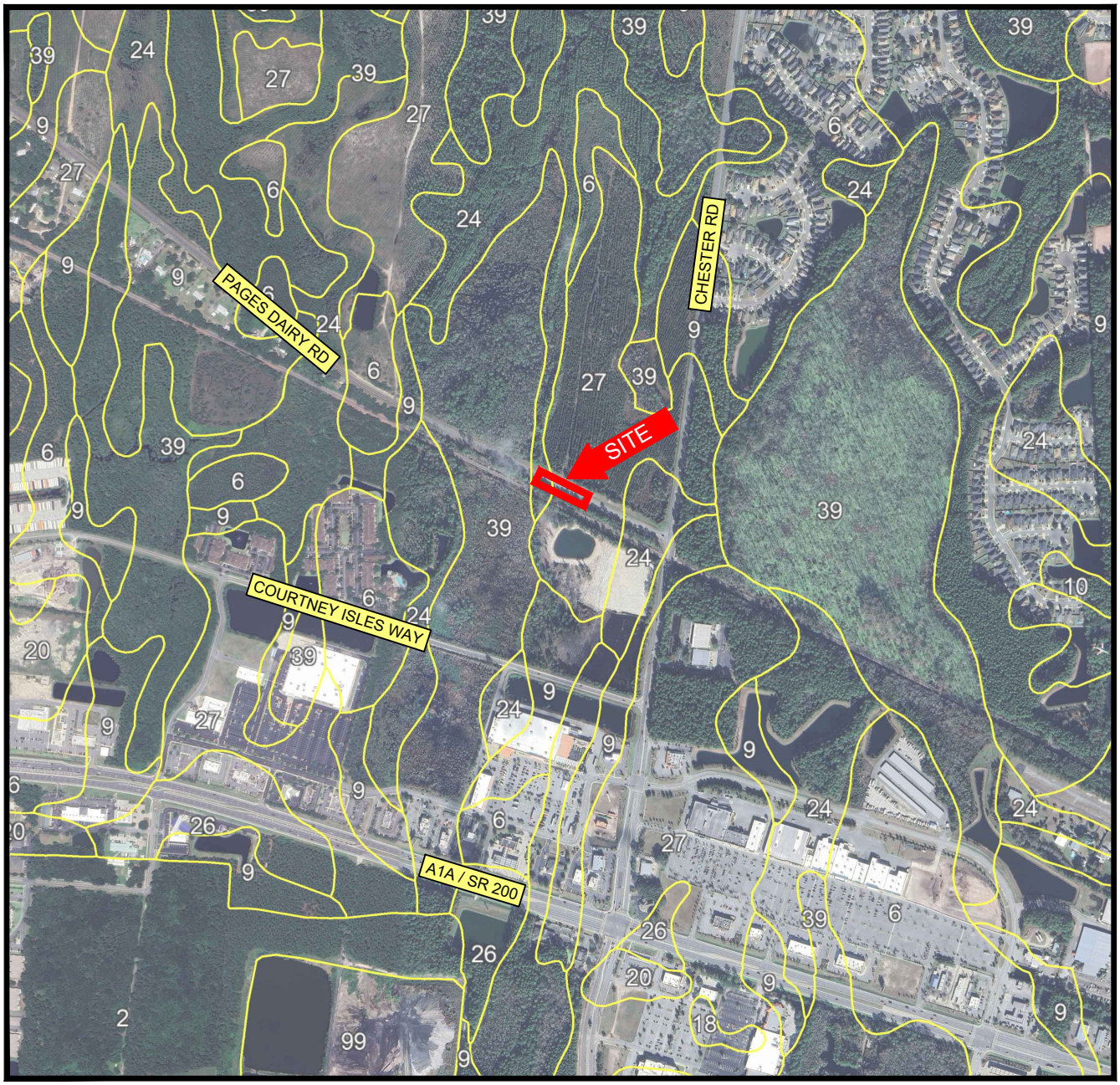
Project Mngr:	TDH
Drawn By:	PJC
Checked By:	TDH
Approved By:	JBK
Project No.	EQ245111
Scale:	AS SHOWN
File No.	EQ245111
Date:	12-9-2024

Terracon
Explore with us
8001 BAYMEADOWS WAY - SUITE 1
JACKSONVILLE, FLORIDA 32256
PH. (904) 900-6494 FAX. (904) 268-5255

TOPOGRAPHIC VICINITY MAP

GEOTECHNICAL ENGINEERING REPORT
PAGES DAIRY ROAD - PAVEMENT CORES

CHESTER ROAD FROM DAVID HALLMAN PARKWAY TO NORTH OF PAGES DAIRY ROAD
YULEE, NASSAU COUNTY, FLORIDA



SCALE 1"=1000'

FEET

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U.S.D.A. SOIL SURVEY FOR NASSAU COUNTY, FLORIDA

SOIL LEGEND

24	KINGSFERRY FINE SAND
27	RIDGEWOOD FINE SAND, 0% TO 5% SLOPES

N

Project Mngr: TDH	Project No. EQ245111	Terracon Explore with us	SOILS MAP
Drawn By: PJC	Scale: AS SHOWN	8001 BAYMEADOWS WAY - SUITE 1 JACKSONVILLE, FLORIDA 32256	GEOTECHNICAL ENGINEERING REPORT PAGES DAIRY ROAD - PAVEMENT CORES
Checked By: TDH	File No. EQ245111	PH. (904) 900-6494 FAX. (904) 268-5255	CHESTER ROAD FROM DAVID HALLMAN PARKWAY TO NORTH OF PAGES DAIRY ROAD YULEE, NASSAU COUNTY, FLORIDA
Approved By: JBK	Date: 12-9-2024		

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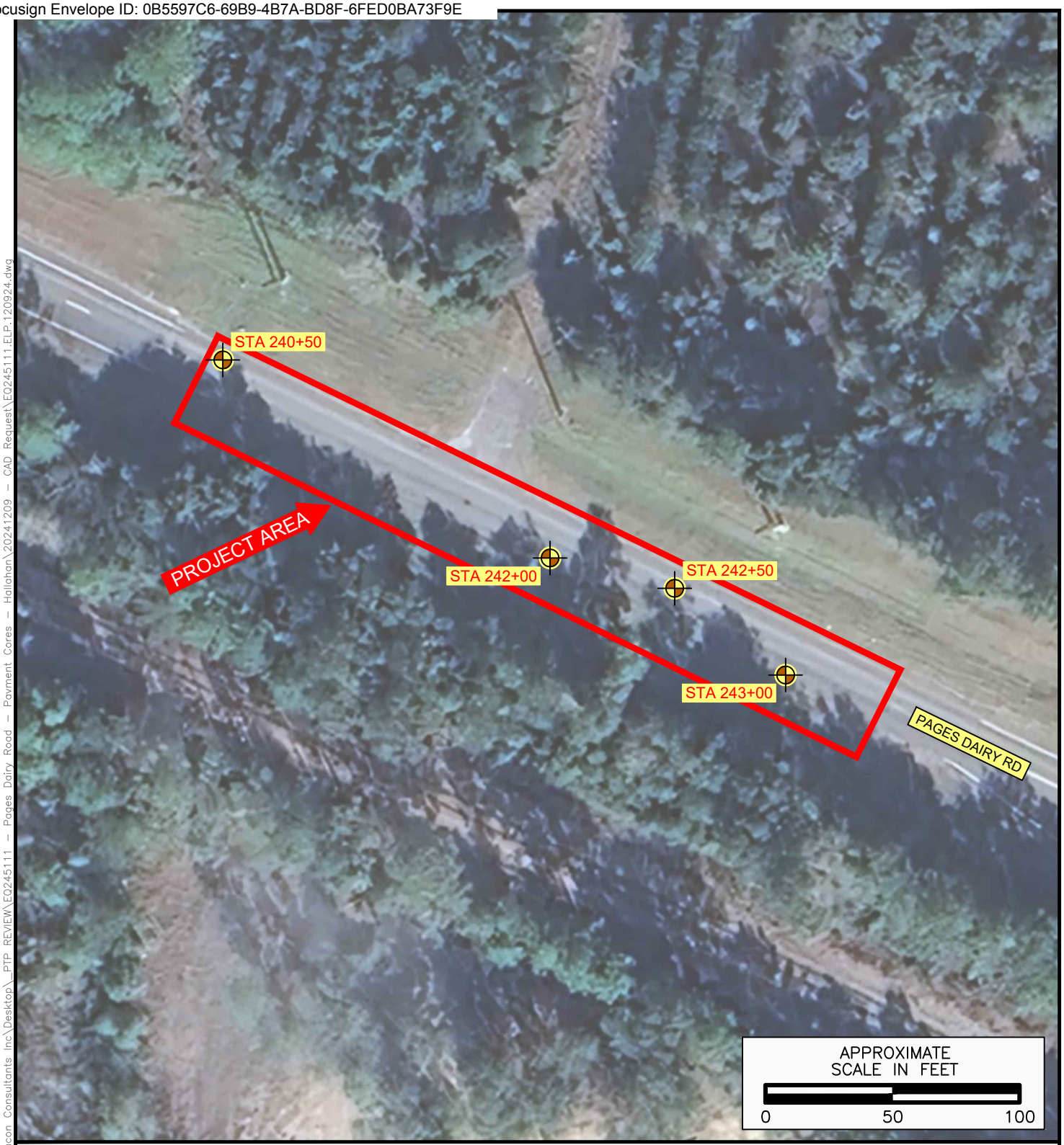


LEGEND

APPROXIMATE LOCATION OF PAVEMENT CORE

Project Mngr:	TDH
Drawn By:	PJC
Checked By:	TDH
Approved By:	JBK

Project No.	EQ245111
Scale:	AS SHOWN
File No.	EQ245111
Date:	12-9-2024

Terracon
Explore with us
8001 BAYMEADOWS WAY - SUITE 1
JACKSONVILLE, FLORIDA 32256
PH. (904) 900-6494 FAX. (904) 268-5255

EXPLORATION LOCATION PLAN

GEOTECHNICAL ENGINEERING REPORT
PAGES DAIRY ROAD - PAVEMENT CORES

CHESTER ROAD FROM DAVID HALLMAN PARKWAY TO NORTH OF PAGES DAIRY ROAD
YULEE, NASSAU COUNTY, FLORIDA

Field Exploration Procedures

Coring Layout: The pavement core locations were provided by Nassau County and were laid out in the field by Terracon personnel, using a GPS hand-held unit, and/or measuring from existing features.

Pavement Cores: The four roadway locations were selected for coring through the existing pavement to determine the asphalt and any base layer thicknesses. The core was performed by hydraulically turning a 4-inch diamond tipped core barrel through the existing pavement until reaching the subgrade soils beneath. The asphalt core was retrieved from the barrel and transported to the laboratory for visual classification. For specific information relative to the encountered pavement conditions, please refer to the Coring Logs in Appendix A.

Hand Augers: The field exploration also consisted of performing hand auger borings beneath the pavement core to an approximate depth below existing grade of 5 feet. The standard hand auger boring procedure consists of manually turning a 3-inch diameter, 6-inch-long bucket type auger sampler into the soil until full. The sampler is then retrieved and the soils in the sampler visually examined and classified. This procedure is repeated until the desired termination depth is achieved. Samples of representative strata are obtained for further visual examination and classification in the laboratory.

Portions of the samples from the borings were sealed in jars to reduce moisture loss, and then the jars were taken to our laboratory for further observation and classification. Upon completion, the boreholes were backfilled with soil cuttings and capped with cold-patch asphalt.

Field logs of each boring were prepared by the drill crew. These logs included visual classifications of the materials encountered during drilling as well as the driller's interpretation of the subsurface conditions between samples.

Maintenance-of-Traffic: The pavement cores were performed in the travel lanes of Pages Dairy Road necessitating MOT lane closures around the coring locations. The lane closure was set up in general accordance with the appropriate Standard Plans Index during drilling operations and subsequently removed once those operations were completed.

Chester Road from David Hallman Parkway to North of Pages Dairy Road | Yulee, FL
Terracon Project No. EQ245111

Terracon
8001 Baymeadows Way Ste 1
Jacksonville, FL

Coring Log No. C240+50

Location: See Exploration Plan

Latitude: 30.6338° Longitude: -81.5488°

Station: 240+50 Offset: -5

Depth (Ft.)

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Water Content (%)	Percent Fines
0.2 **ASPHALT - 2.5" THICK**					
POORLY GRADED SAND WITH SILT (SP-SM), fine grained, brown, trace fine limerock fragments 0.8					
POORLY GRADED SAND (SP), fine grained, brown	1			6.3	5
below 2 feet - brown to gray	2				
	3				
below 4 feet - gray	4	▽			
5.0	5				
Coring Terminated at 5 Feet					

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
▽ Groundwater encountered at 4.5 feet at time of drilling

Drill Rig
Coring Rig with hand auger

Driller
B. Harpster

Notes

Advancement Method
Hydraulic core barrel with hand auger

Abandonment Method
Boring backfilled with Auger Cuttings
Surface capped with asphalt

Logged by
T. Cozart

Boring Started
11-14-2025

Boring Completed
11-14-2025

Chester Road from David Hallman Parkway to North of Pages Dairy Road | Yulee, FL
Terracon Project No. EQ245111

Terracon
8001 Baymeadows Way Ste 1
Jacksonville, FL

Coring Log No. C242+00

Location: See Exploration Plan

Latitude: 30.6336° Longitude: -81.5484°

Station: 242+00 Offset: 5

Graphic Log	Depth (Ft.)	Water Level Observations	Sample Type	Water Content (%)	Percent Fines
	0.2 **ASPHALT - 2.3" THICK**				
	POORLY GRADED SAND WITH SILT (SP-SM), fine grained, brown, few coarse to fine limerock fragments 0.8				
	POORLY GRADED SAND WITH SILT (SP-SM), fine grained, brown 2.0				
	POORLY GRADED SAND (SP), fine grained, light brown to gray				
	below 3 feet - gray			16.1	2
	5.0	▽			
	Coring Terminated at 5 Feet				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
▽ Groundwater encountered at 5.0 feet at time of drilling

Drill Rig
Coring Rig with hand auger

Driller
B. Harpster

Notes

Advancement Method
Hydraulic core barrel with hand auger

Logged by
T. Cozart

Abandonment Method
Boring backfilled with Auger Cuttings
Surface capped with asphalt

Boring Started
11-14-2024

Boring Completed
11-14-2024

Terracon
8001 Baymeadows Way Ste 1
Jacksonville, FL

Coring Log No. C242+50

Location: See Exploration Plan

Latitude: 30.6335° Longitude: -81.5482°

Station: 242+50 Offset: -5

Graphic Log	Depth (Ft.)	Material Description	Depth (Ft.)	Water Level Observations	Sample Type	Water Content (%)	Percent Fines
	0.2	**ASPHALT - 2.6" THICK**					
	1.0	**POORLY GRADED SAND WITH SILT (SP-SM)**, fine grained, brown, few coarse to fine limerock fragments	1				
	2.5	**POORLY GRADED SAND WITH SILT (SP-SM)**, fine grained, brown to light brown	2			5.6	6
	5.0	**POORLY GRADED SAND (SP)**, fine grained, gray	3				
		Coring Terminated at 5 Feet	4				
			5				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered to termination depth

Drill Rig
Coring Rig with hand auger

Driller
B. Harpster

Notes

Advancement Method
Hydraulic core barrel with hand auger

Abandonment Method
Boring backfilled with Auger Cuttings
Surface capped with asphalt

Logged by
T. Cozart

Boring Started
11-14-2024

Boring Completed
11-14-2024

Chester Road from David Hallman Parkway to North of Pages Dairy Road | Yulee, FL
Terracon Project No. EQ245111

8001 Baymeadows Way Ste 1
Jacksonville, FL

Coring Log No. C243+00

Location: See Exploration Plan

Latitude: 30.6334° Longitude: -81.5481°

Station: 243+00 Offset: 5

Graphic Log	Depth (Ft.)		Water Level Observations	Sample Type	Water Content (%)	Percent Fines
	0.2	**ASPHALT - 2.6" THICK**				
	0.8	**SILTY SAND (SM)**, fine grained, brown, few coarse to fine limerock fragments			10.7	13
	2.5	**POORLY GRADED SAND WITH SILT (SP-SM)**, fine grained, brown				
	5.0	**POORLY GRADED SAND (SP)**, fine grained, brown to gray				
		Coring Terminated at 5 Feet				

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Water Level Observations
Groundwater not encountered to termination depth

Drill Rig
Coring Rig with hand auger

Driller
B. Harpster

Notes

Advancement Method
Hydraulic core barrel with hand auger

Abandonment Method
Boring backfilled with Auger Cuttings
Surface capped with asphalt

Logged by
T. Cozart

Boring Started
11-14-2024

Boring Completed
11-14-2024

Appendix B

Laboratory Testing Procedures

Summary of Laboratory Testing

Laboratory Testing Procedures

General: During the field exploration, a portion of each recovered soil sample was placed in glass jars and transported to our laboratory for further visual observation and laboratory testing. Representative samples were tested for moisture content, organic content, grain size distribution and fines content. The visual-manual classifications for soil samples were modified as appropriate based upon the laboratory testing results.

The soil samples were classified in general accordance with the Unified Soil Classification System and AASHTO soil classification system based on the material's texture and plasticity. A brief description of the AASHTO and Unified Soil Classification System is included in Appendix C. The following are brief descriptions of the laboratory testing procedures.

Moisture Content: To determine the moisture content of the selected soil sample, the test specimen was dried in an oven to constant mass in general accordance with AASHTO T 265. The water content was then calculated using the mass of the water and the mass of the dry specimen. The water content is used to express the phase relationship of air, water, and solid in a given volume of material.

Grain Size Distribution: To conduct this test, a sample is dried and then shaken over various standard sieve sizes. The weight of soil retained on each sieve is measured and the cumulative percentage by weight passing each sieve is calculated. This test was conducted in general accordance with AASHTO T 088.

Grain Size Distribution – Fines Content: To conduct this test, a sample is dried and then washed over a No. 200 Standard U.S. sieve to remove the soil particles finer than 0.075 millimeters in diameter (I.e. silt and clay size particles). The soil retained on the sieve is then decanted and dried to determine the dry weight retained on the sieve which is then subtracted from the original dry weight to determine the fines content of the sample expressed as a percentage. This test was conducted in general accordance with AASHTO T 088.

EXHIBIT B-2
SUMMARY OF LABORATORY TESTING
PAGES DAIRY ROAD – PAVEMENT CORES
YULEE, NASSAU COUNTY, FLORIDA
TERRACON PROJECT NO. EQ245111

Coring No.	Depth (Feet)	Moisture Content (%)	Percent Passing Sieve No.						Unified Soil Type	AASHTO Soil Type
			4	10	40	60	100	200		
C240+50	0.8 – 2.0	6.3	100	100	99	96	54	4.6	SP	A-3
C242+00	3.0 – 5.0	16.1	100	100	100	98	56	1.9	SP	A-3
C242+50	1.0 – 1.5	5.6	100	100	100	97	51	5.5	SP-SM	A-3
C243+00	0.2 – 0.8	10.7	91	87	80	76	46	13.1	SM	A-2-4

Appendix C

AASHTO Soil Classification System

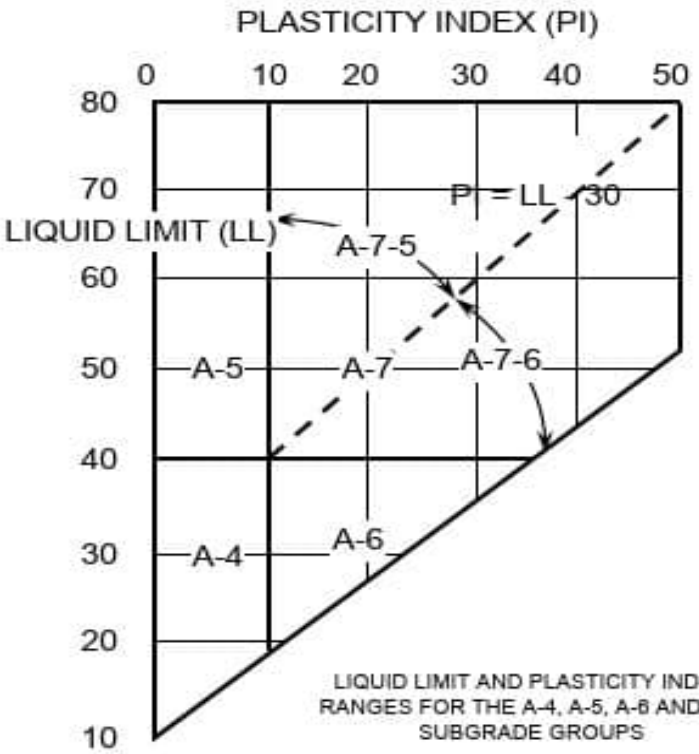
Unified Soil Classification System

General Notes

AASHTO Soil Classification System

Group	Subgroup	Percent Passing U.S. Sieve No.			Character of Fraction Passing No. 40 Sieve		Group Index No.[3]
		10	40	200	Liquid Limit (LL)	Plasticity Index (PI)	
A-1			50 Max.	25 Max.	-	6 Max.	0
	A-1-a	50 Max.	30 Max.	15 Max.	-	6 Max.	0
	A-1-b		50 Max.	25 Max.	-	6 Max.	0
A-2[1]				35 Max.			0 to 4
	A-2-4			35 Max.	40 Max.	10 Max.	0
	A-2-5			35 Max.	41 Min.	10 Max.	0
	A-2-6			35 Max.	40 Max.	11 Min.	4 Max.
	A-2-7			35 Max.	41 Min.	11 Min.	4 Max.
A-3	-		51 Min.	10 Max.	-	Non-Plastic	0
A-4	-	-	-	36 Min.	40 Max.	10 Max.	8 Max.
A-5	-	-	-	36 Min.	41 Min.	10 Max.	12 Max.
A-6	-	-	-	36 Min.	40 Max.	11 Min.	16 Max.
A-7[2]				36 Min.	41 Min.	11 Min.	20 Max.
	A-7-5			36 Min.	41 Min.	11 Min.	20 Max.
	A-7-6			36 Min.	41 Min.	11 Min.	20 Max.
A-8	HIGHLY ORGANIC SOIL (Qualifying Minimum Organic Content Varies by Region – Typically > 5% by Weight)						

1. Group A-2 includes all soils having 35% or less passing the #200 sieve that cannot be classified as A-1 or A-3
2. PI of A-7-5 subgroup is equal to or less than LL – 30. PI of A-7-6 subgroup is greater than LL - 30
3. Group Index GI = (F – 35)[0.2 + 0.005(LL – 40)] + 0.01 (F – 15)(PI – 10) where F = % passing #200 sieve.

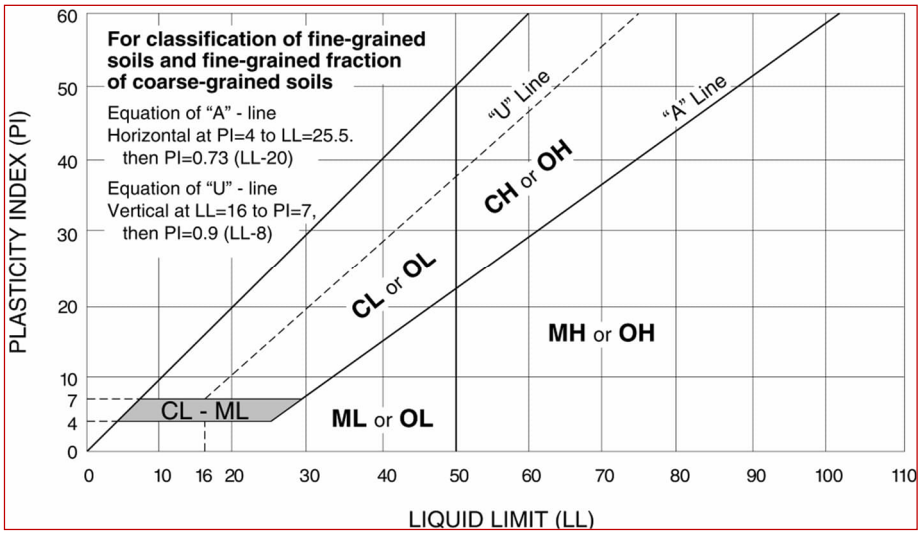


LIQUID LIMIT AND PLASTICITY INDEX RANGES FOR THE A-4, A-5, A-6 AND A-7 SUBGRADE GROUPS

Unified Soil Classification System

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests [A]				Soil Classification	
				Group Symbol	Group Name [B]
Coarse-Grained Soils: More than 50% retained on No. 4 sieve	Gravels: More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels: Less than 5% fines [C]	Cu≥4 and 1≤Cc≤3 [E]	GW	Well-graded gravel [F]
			Cu<4 and/or [Cc<1 or Cc>3.0] [E]	GP	Poorly graded gravel [F]
		Gravels with Fines: More than 12% fines [C]	Fines classify as ML or MH	GM	Silty gravel [F, G, H]
			Fines classify as CL or CH	GC	Clayey gravel [F, G, H]
	Sands: 50% or more of coarse fraction passes No. 4 sieve	Clean Sands: Less than 5% fines [D]	Cu≥6 and 1≤Cc≤3 [E]	SW	Well-graded sand [I]
			Cu<6 and/or [Cc<1 or Cc>3.0] [E]	SP	Poorly graded sand [I]
		Sands with Fines: More than 12% fines [D]	Fines classify as ML or MH	SM	Silty sand [G, H, I]
			Fines classify as CL or CH	SC	Clayey sand [G, H, I]
Fine-Grained Soils: 50% or more passes the No. 200 sieve	Silts and Clays: Liquid limit less than 50	Inorganic:	PI > 7 and plots above "A" line [J]	CL	Lean clay [K, L, M]
			PI < 4 or plots below "A" line [J]	ML	Silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried}$ < 0.75	OL	Organic clay [K, L, M, N]
					Organic silt [K, L, M, O]
	Silts and Clays: Liquid limit 50 or more	Inorganic:	PI plots on or above "A" line	CH	Fat clay [K, L, M]
			PI plots below "A" line	MH	Elastic silt [K, L, M]
		Organic:	$\frac{LL\ oven\ dried}{LL\ not\ dried}$ < 0.75	OH	Organic clay [K, L, M, P]
					Organic silt [K, L, M, Q]
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat

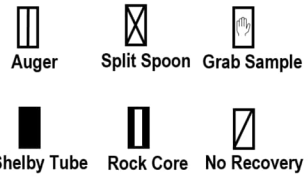
[A] Based on the material passing the 3-inch (75-mm) sieve.
[B] If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.
[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.
[E] $Cu = D_{60}/D_{10}$ $Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$
[F] If soil contains ≥ 15% sand, add "with sand" to group name.
[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.
[H] If fines are organic, add "with organic fines" to group name.
[I] If soil contains ≥ 15% gravel, add "with gravel" to group name.
[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.
[K] If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.
[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.
[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.
[N] PI ≥ 4 and plots on or above "A" line.
[O] PI < 4 or plots below "A" line.
[P] PI plots on or above "A" line.
[Q] PI plots below "A" line.



General Notes

DESCRIPTION OF SYMBOLS AND ABBREVIATIONS

SAMPLING



WATER LEVEL

Water Initially Encountered

Water Level After a Specified Period of Time

Water Level After a Specified Period of Time

Water levels indicated on the soil boring logs are the levels measured in the borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils, accurate determination of groundwater levels is not possible with short-term water level observations.

FIELD TESTS

(HP)	Hand Penetrometer
(T)	Torvane
(b/f)	Standard Penetration Test (blows per foot)
(PID)	Photo-Ionization Detector
(OVA)	Organic Vapor Analyzer

DESCRIPTIVE SOIL CLASSIFICATION: Soil classification is based on the United Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on basis of their in-place relative density and fine-grained soils on the basis of their consistency.

LOCATION AND ELEVATION NOTES: Unless otherwise noted, Latitude and Longitude are approximately determined using a handheld GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topography survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

RELATIVE DENSITY OF COARSE-GRAINED SOILS (More that 50% retained on No. 200 sieve) Density determined by Standard Penetration Resistance			CONSISTENCY OF FINE-GRAINED SOILS (50% or more passing the No. 200 sieve) Consistency determined by laboratory shear strength testing, field visual-manual procedures or Standard Penetration Resistance			
Descriptive Term (Density)	Safety Hamer SPT N_{60}-Value (Blows/Ft.)	Automatic Hammer SPT N-Value (Blows/Ft.)	Descriptive Term (Consistency)	Unconfined Compressive Strength Qu, (psf)	Safety Hamer SPT N_{60}-Value (Blows/Ft.)	Automatic Hamer SPT N-Value (Blows/Ft.)
Very Loose	0 – 3	< 3	Very Soft	Less than 500	0 – 1	< 1
Loose	4 – 9	3 – 8	Soft	500 – 1,000	2 – 4	1 – 3
Medium Dense	10 – 29	9 – 24	Medium Stiff	1,000 – 2,000	5 – 8	4 – 6
Dense	30 – 50	24 – 40	Stiff	2,000 – 4,000	9 – 15	7 – 12
Very Dense	> 50	> 40	Very Stiff	4,000 – 8,000	16 – 30	13 – 24
			Hard	> 8,000	> 30	> 24

RELATIVE PROPORTIONS OF GRAVEL, SAND, FINES OR ORGANIC MATTER	
Descriptive Terms of Constituents	Percent of Dry Weight
Trace	< 5%
Few	5 to < 12%
Little	12 to < 30%
Some	30 to < 50%
Mostly	>50%

GRAIN SIZE TERMINOLOGY	
Major Component of Sample	Particle Size
Boulders	Over 12 in. (300mm)
Cobbles	12 in. to 3 in. (300mm to 75mm)
Gravel	3 in. to #4 sieve (75mm to 4.75mm)
Sand	#4 to #200 sieve (4.75mm to 0.075mm)
Silt or Clay	Passing #200 sieve (0.075mm)

PLASTICITY DESCRIPTION	
Term	Plasticity Index
Non-Plastic	0
Low Plasticity	1-10
Medium Plasticity	11-30
High Plasticity	>30

ORGANIC SOIL CLASSIFICATION	
Classification/Modifier	Conditions
Trace Organic Matter (SP, SP-SM, SP-SC, SM, SC, ML, CL, MH, CH)	< 5% Organic Content By Weight
Organic Sand (SP), Organic Silty Sand (SM), Organic Clayey Sand (SC)	5% to < 30% Organic Content by Weight
Organic Silt (OL) or Organic Clay (OH)	> 50% Passing #200 sieve and 5 to < 30% Organic Content by Weight
PEAT (Pt)	30% to 100% Organic Content by Weight

ROCK CEMENTATION (NORTH FLORIDA LIMESTONE) CORRELATION OF LABORATORY UNCONFINED COMPRESSION TEST RESULTS AND PENETRATION RESISTANCE WITH RELATIVE DEGREE OF CEMENTATION			
Relative Degree of Cementation	**Unconfined Compressive Strength (ksf)**	**Standard Penetration Resistance (# of Blows, N_{60})**	**Manual Test on Recovered Core**
Weakly Cemented	Uc < 50	N < 100	Friable, easily crumbled or broken with hands
Cemented	50 < Uc < 250	N – 100 or more w/recovery	Some difficulty in breaking with hands
Well Cemented	Uc > 250	No recovery – N>100	Cannot be broken with hands

CORRELATION OF RATE OF EFFERVESCENCE OF DILUTE HYDROCHLORIC ACID WITH RELATIVE DEGREE OF CALCAREOUSNESS	
Relative Degree of Calcareousness	Rate of Effervescence
Slightly Calcareous	Weak or Slow
Calcareous	Moderate or Mild
Very Calcareous	Strong or Violent

Certificate Of Completion

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Source Envelope:
Document Pages: 31
Signatures: 5
Envelope Originator:
Certificate Pages: 6
Initials: 2
George Murray
AutoNav: Enabled
gmurray@nassaucountyfl.com
EnvelopeId Stamping: Enabled
IP Address: 50.238.237.26
Time Zone: (UTC-05:00) Eastern Time (US & Canada)

Record Tracking

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Holder: George Murray
gmurray@nassaucountyfl.com
Location: DocuSign

Signer Events	Signature	Timestamp
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Tracy Poore tpoore@nassaucountyfl.com OMB Admin Nassau County BOCC Security Level: Email, Account Authentication (None)	 Signature Adoption: Pre-selected Style Using IP Address: 50.238.237.26	Sent: 3/6/2025 12:33:28 PM Viewed: 3/6/2025 12:58:14 PM Signed: 3/6/2025 1:03:52 PM
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chris lacambra clacambra@nassaucountyfl.com OMB Director Nassau County BOCC Security Level: Email, Account Authentication (None)	 Signature Adoption: Pre-selected Style Using IP Address: 50.238.237.26	Sent: 3/6/2025 1:03:54 PM Viewed: 3/6/2025 4:33:59 PM Signed: 3/7/2025 4:40:54 PM
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Lanaee Gilmore lgilmore@nassaucountyfl.com Procurement Director Nassau County BOCC Security Level: Email, Account Authentication (None)	 Signature Adoption: Pre-selected Style Using IP Address: 50.238.237.26	Sent: 3/7/2025 4:40:57 PM Viewed: 3/11/2025 9:15:14 AM Signed: 3/11/2025 9:15:20 AM
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Abigail Jorandby ajorandby@nassaucountyfl.com Deputy County Attorney Nassau BOCC Security Level: Email, Account Authentication (None) **Electronic Record and Signature Disclosure:** Not Offered via Docusign	 Signature Adoption: Pre-selected Style Using IP Address: 50.238.237.26	Sent: 3/11/2025 9:15:24 AM Viewed: 3/13/2025 12:14:05 PM Signed: 3/13/2025 12:15:28 PM
Denise C. May, Esq., BCS dmay@nassaucountyfl.com County Attorney Nassau County BOCC Security Level: Email, Account Authentication (None) **Electronic Record and Signature Disclosure:** Not Offered via Docusign	 Signature Adoption: Pre-selected Style Using IP Address: 50.238.237.26	Sent: 3/13/2025 12:15:31 PM Viewed: 3/13/2025 12:28:29 PM Signed: 3/13/2025 12:29:19 PM
Taco Pope, AICP tpope@nassaucountyfl.com County Manager Nassau County BOCC Security Level: Email, Account Authentication (None) **Electronic Record and Signature Disclosure:** Not Offered via Docusign	 Signature Adoption: Drawn on Device Using IP Address: 50.238.237.26	Sent: 3/13/2025 12:29:23 PM Viewed: 3/13/2025 2:25:31 PM Signed: 3/13/2025 2:26:52 PM

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Certified Delivery Events	Status	Timestamp

Carbon Copy Events	Status	Timestamp
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Witness Events	Signature	Timestamp

Notary Events	Signature	Timestamp

Envelope Summary Events	Status	Timestamps
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Completed	Security Checked	3/13/2025 2:26:56 PM

Payment Events	Status	Timestamps

Electronic Record and Signature Disclosure

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You may contact us to let us know of your changes as to how we may contact you electronically, to request paper copies of certain information from us, and to withdraw your prior consent to receive notices and disclosures electronically as follows:
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To let us know of a change in your email address where we should send notices and disclosures electronically to you, you must send an email message to us at bsimmons@nassaucountyfl.com and in the body of such request you must state: your previous email address, your new email address. We do not require any other information from you to change your email address.

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ii. send us an email to bsimmons@nassaucountyfl.com and in the body of such request you must state your email, full name, mailing address, and telephone number. We do not need any other information from you to withdraw consent.. The consequences of your withdrawing consent for online documents will be that transactions may take a longer time to process..

Required hardware and software

The minimum system requirements for using the DocuSign system may change over time. The current system requirements are found here: https://support.docusign.com/guides/signer-guide-signing-system-requirements.

Acknowledging your access and consent to receive and sign documents electronically

To confirm to us that you can access this information electronically, which will be similar to other electronic notices and disclosures that we will provide to you, please confirm that you have read this ERSD, and (i) that you are able to print on paper or electronically save this ERSD for your future reference and access; or (ii) that you are able to email this ERSD to an email address where you will be able to print on paper or save it for your future reference and access. Further, if you consent to receiving notices and disclosures exclusively in electronic format as described herein, then select the check-box next to 'I agree to use electronic records and signatures' before clicking 'CONTINUE' within the DocuSign system.

By selecting the check-box next to 'I agree to use electronic records and signatures', you confirm that:

- You can access and read this Electronic Record and Signature Disclosure; and
- You can print on paper this Electronic Record and Signature Disclosure, or save or send this Electronic Record and Disclosure to a location where you can print it, for future reference and access; and
- Until or unless you notify County of Nassau as described above, you consent to receive exclusively through electronic means all notices, disclosures, authorizations, acknowledgements, and other documents that are required to be provided or made available to you by County of Nassau during the course of your relationship with County of Nassau.